GENERAL NOTES

1. ALL WORK SHALL COMPLY WITH INTERNATIONAL BUILDING CODE, 2012 EDITION.

2. ALL SYMBOLS AND ABBREVIATIONS USED ON THE DRAWINGS ARE CONSIDERED TO BE CONSTRUCTION STANDARDS. IF CLARIFICATION IS REQUIRED, THE CONTRACTOR SHALL NOTIFY THE CLIENT PRIOR TO PROCEEDING WITH THE WORK.

3. ALL DIMENSIONS AND THE SITE CONDITIONS SHALL BE VERIFIED BY THE CONTRACTOR AT THE JOB SITE, START OF SHOP DRAWINGS, START OF CONSTRUCTION, AND/OR FABRICATION OF MATERIALS. IF DISCREPANCIES ARE ENCOUNTERED, OR CONDITIONS DEVELOP NOT COVERED BY THE CONTRACT DOCUMENTS, THE CONSULTING TEAM SHALL BE NOTIFIED FOR CLARIFICATION.

4. CONTRACTOR SHALL PROVIDE AND BE RESPONSIBLE FOR THE PROTECTION AND REPAIR OF ADJACENT EXISTING SURFACES AND AREAS WHICH MAY BE DAMAGED AS A RESULT OF NEW WORK.

5. DO NOT SCALE DRAWINGS. PRINTED DIMENSIONS HAVE PRECEDENCE OVER SCALED DRAWINGS AND LARGE SCALE OVER SMALL.

6. WHERE NO SPECIFIC DETAIL IS SHOWN THE FRAMING OR CONSTRUCTION SHALL BE IDENTICAL OR SIMILAR TO THAT INDICATED FOR LIKE CASES OF CONSTRUCTION ON THIS PROJECT.

7. THE CONSTRACT DOCUMENTS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE THEY DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONSTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE STURCTURE AND SAFETY OF WORKMEN DURING CONSTRUCTION. SUCH MEASURES SHALL INCLUDE, BUT NOT BE LIMITED TO, BRACING, SHORING FOR LOADS DUE TO CONSTURCTION EQUIPMENT, ETC. OBSERVATION VISITS TO THE SITE BY THE CLIENT OR STRUCTURAL ENGINEER SHALL NOT INCLUDE INSPECTION OF THE ABOVE ITEMS AND DOES NOT IN ANY WAY RELIEVE THE CONTRACTOR OF HIS RESPONSIBILITIES FOR THE ABOVE.

8. NO HOLES, NOTCHES, BLOCKOUTS, ETC. ARE ALLOWED IN STRUCTURAL ELEMENTS UNLESS DETAILED ON THE STRUCTURAL DRAWINGS OR APPROVED BY THE STRUCTURAL ENGINEER.

9. COORDINATE BUILDING FOOTPRINT WITH ARCH. DWGS FOR DIMENSIONS.

10. IN CASE OF DISCREPENCIES BETWEEN NOTES & SPECIFICATIONS, THESE NOTES SHALL TAKE PRECEDENCE OVER SPECIFICATIONS.

11. CUTTING, BORING. SAWCUTTING OR DRILLING THROUGH THE NEW OR EXISTING STRUCTURAL ELEMENTS TO BE DONE ONLY WHEN SHOWN IN DETAIL ON THE DRAWINGS OR ACCEPTED BY THE ARCHITECT (STRUCTURAL ENGINEER) WITH THE APPROVED OF BUILDING OFFICIALS & GOVERNING CODE AUTHORITIES.

12. THE EXACT DEPTH, EXTENT, AND LOCATION OF ALL FLOOR DEPRESSIONS, ELEVATED AREAS, OR OTHER IRREGULARITIES SHALL BE COORDINATED WITH ARCHITECTURAL OR APPLICABLE DRAWINGS. THE STRUCTURAL DRAWINGS DO NOT NECESSARILY INDICATED ALL OF THESE ITEMS.

DESIGN LOADS

THE SEISMIC DESIGN CRITERIA:

ACCOMODATION BUILDINGS AND NON ESSENTIAL FACILITIES:
I = 1.0
OCCUPANCY CATEGORY = II
SEISMIC DESIGN CATEGORY = D
SPECIAL REINFORCED MASONRY BEARING/ SHEAR WALLS: R = 5 Cd = 3.5 Ω = 2.5

CRITICAL BUILDINGS:
I = 1.5
OCCUPANCY CATEGORY = IV
SEISMIC DESIGN CATEGORY = D
SPECIAL REINFORCED MASONRY BEARING/ SHEAR WALLS: R = 5 Cd = 3.5 Ω = 2.5

BASED ON REGION PARAMETERS FOR BASRA, MCE, SOIL CLASS - B
Ss = 0.98, S1 = 0.39 (MAPPED SPECTRAL ACCELERATION AT MCE)
SITE CLASS = E
Fa = 0.90, Fv = 2.40 (SITE AMPLIFICATION FACTORS, ASCE-7, TABLE 11.4-1&2)
SMs = 0.89, SM1 = 0.94 (SITE ADJUSTED MCE SPECTRAL ACCELERATIONS)
SDs = 0.59, SD1 = 0.62 (DESIGN SPECTRAL ACCELERATIONS)

SITE-SPECIFIC PEAK GROUND ACCELERATION

ASSUMING AN DESIGN LEVEL
SDS = PGAx2.5 = 0.384x2.5 = **0.96** (DESIGN SPECTRAL ACCELERATION AT PLATEAU) **GOVERNS**

ASSUMING AN MCE LEVEL
SDS = PGAx2.5 = 0.384x2/3x2.5 = 0.64 (DESIGN SPECTRAL ACCELERATION AT PLATEAU)

WIND LOADING

DESIGN WIND SPEED = 100 MPH (160Km/H)
WIND IMPORTANCE FACTOR = 1.0
EXPOSURE = C (FLAT UNOBSTRUCTED)
BASIC WIND PRESSURE AT THE ROOF LEVEL = 11 PSF (530 Pa) (SINGLE-STORY NON ESSENTIAL FACILITY)

DEAD LOAD (PRE-ENGINEERED ROOF SYSTEM)

ROOF = 25 PSF (1.2 kPa) (NOT-TO-EXCEED)

LIVE LOAD

ROOF = 20 PSF (1.0 kPa) (REDUCIBLE)

EARTHWORK AND FOUNDATION

1. ALLOWABLE FOUNDATION DESIGN VALUES PER 2009 IBC TABLE 1806.2: VALUES BELOW MAY BE INCREASED 33 PERCENT FOR TANSIENT LOADING.

A. BEARING CAPACITY:
i. GROUND LEVEL, ALL FOOTING TYPES.....**1,500 PSF (10 MPa)**

2. GRADING, EXCAVATIONS, BACKFILL AND COMPACTION OF BACKFILL: COMPLY WITH GEOTECHNICAL REPORT AND REQUIREMENTS OF GOVERNING CODE AUTHORITY.

3. PREPARATION OF SOIL UNDER PAD: SEE GEOTECHNICAL REPORT FOR OVER-EXCAVATION OF EXISTING SOIL AND INSTALLATION OF PROPERLY COMPACTED BACKFILL.

4. FOUNDATION EXCAVATION: FOUNDATIONS ARE TO BEAR ON APPROVED COMPACTED FILL AS INDICATED IN GEOTECHNICAL REPORT. ENSURE EXCAVATIONS ARE CLEAN, DRY AND FREE OF DEBRIS OR LOOSE SOIL. SLOPE SIDES OF EXCAVATION NOT LESS THAN MINIMUM SLOPE INDICATED IN GEOTECHNICAL REPORT. CAST CONCRETE DIRECTLY AGAINST EXCAVATED SURFACES.

5. BACKFILLING OF RETAINING WALLS: PLACE AFTER COMPLETION AND INSPECTION OF WATERPROFING. ADEQUATELY SHORE RETAINING WALLS DUCRING BACKFILL OPERATION. UNLESS ADEQUATELY SHORED, DO NOT PLACE BACKFILL BEHIND STRUCTURE RETAINING WALLS (EXCLUDING SITE RETAINING WALLS) UNTILL CONCRETE AT ELEVATED FLOOR LEVELS ADJACENT TO WALLS ARE COMPLETELY POURED AND HAVE CURED FOR AT LEAST 7 DAYS. APPROVED CURING COMPOUNDS MAY BE USED.

CAST-IN-PLACE CONCRETE

1. APPLICABLE STANDARDS: ACI 318 AND ACI A301 EXCEPT AS MODIFIED BY SUPPLEMENTAL REQUIREMENTS HEREIN.

2. PORTLAND CEMENT: ASTM C150, TYPE V

3. AGGREGATES:
A. NORMAL WEIGHT CONCRETE AGGREGATE: ASTM C33 FOR AGGREGATES OF NATURAL SAND AND ROCK. MAXIMUM AGGREGATE SIZE IS 1-1/2 INCHES (40mm) AT FOUNDATIONS AND SLABS ON GRADE AND 1"(25mm) ELSEWHERE.
B. LIGHT WEIGHT AGGREGATE FOR STRUCTURAL CONCRETE: ASTM C330, EXPANDED SHALE LIGHT WEIGHT AGGREGATES OF PEA GRAVEL SIZE.

4. MINIMUM 28-DAY CONCRETE COMPRESSIVE STRENGTH AND TYPES FOR NORMAL WEIGHT (145PCF/ 2325kg/CUBIC METER

LOCATION IN STRUCTURE	COMPRESSIVE STRENGTH CUBICAL SAMPLES	MAX W/C RATIO
CONCRETE UNLESS OTHERWISE INDICATED ON STRUCTURAL DRAWINGS	3500 PSI (23 MPa)	0.5
FOOTINGS AND GRADE BEAMS	4500 PSI (31 MPa)	0.5
COLUMNS, PILASTERS, AND BEAMS	4500 PSI (31 MPa)	0.5
CONCRETE SLABS ON GRADE	3500 PSI (23 MPa)	0.5

5. LEAN CONCRETE: WHERE SPECIFICALLY INDICATED, CONTAINING 2 SACKS OF CEMENT PER CUBIC YARD OF CONCRETE.

6. NON-SHRINK GROUT: ASTM C109, CAMENTITIOUS, NON-METALLIC ATTAINING A COMPRESSIVE STRENGTH OF 6000 PSI (41 MPa)

7. CONCRETE MIX DESIGN SUBMITTAL: PRIOR TO ORDERING CONCRETE, SUBMIT FOR EACH COMPRESSIVE STRENGTH AND TYPE OF CONCRETE REQUIRED DESIGNED

8. CONSTRUCTION JOINT SUBMITTAL: NO CONSTRUCTION JOINTS ARE ALLOWED OTHER THAN THE ONES SHOWN ON THESE DRAWINGS.

9. EMBEDDED PIPES, SLEEVES, CONDUITS, AND DUCTS: NOT PERMITTED EMBEDDED OR PENETRATING CONCRETE UNLESS SPECIFICALLY ACCEPTANCE IN WRITING BY ARCHITECT (STRUCTURAL ENGINEER).
A. PIPES, SLEEVES, CONDUITS, AND DUCTS: NOT PERMITTED EMBEDDED OR PENETRATING CONCRETE SPREAD FOOTINGS, COLUMNS, WALLS OR CONCRETE CAST OVER METAL DECKING.
B. CONDUITS EMBEDDED INSTRUCTURAL CONCRETE SLABS: NOT PERMITTED UNLESS LIMITED TO TWO LAYERS OF 1 INCH (25mm) OUTSIDE DIAMETER CONDUITS AND SMALLER SPACED AT LEAST 3 INCHES (75mm) CENTER TO CENTER AND WITHIN MIDDLE THIRD OF SLAB THICKNESS. NO CONDUIT EMBEDDED IN CONCRETE CAST OVER METAL DECKING IS PERMITTED.

10. CHAMFERED CORNERS: PROVIDE ¾-INCH (20mm) CHAMFER AT EXPOSED CORNERS OF COLUMNS, BEAMS AND WALLS EXCEPT WHERE STRUCTURAL WALLS ARE LAID FLUSH WITH COLUMN OR BEAM FACES, UNLESS DETAILED OTHERWISE.

11. CONSTRUCTION JOINTS: CONSTRUCTION JOINTS SHALL HAVE ENTIRE SURFACE REMOVED TO MIN 1/4" (10mm) TO EXPOSE CLEAN, SOLIDLY EMBEDDED AGGREGATE PER TYP. DETAILS, PROVIDED IN THIS SET. CONSTRUCTION JOINTS SHALL BE PROVIDED TO LIMIT SHRINKAGE CRACKS. A MAX DISTANCE OF 50FT (15m). SHALL BE CONSIDERED IF NO CONTROL JOINTS IS CALLED ON PLANS. THE CONTRACTOR SHALL OBTAIN THE ENGINEER'S APPROVAL OF CONSTRUCTION JOINT LOCATION IN SLABS, WALL AND BEAMS. JOINTS SHALL BE SHOWN ON SHOP DRAWING CONFORMING TO THE ABOVE REQUIREMENT.

12. CURING: MAINTAIN CONCRETE ABOVE 60 DEGREES FAHRENHEIT (10 DEG. CELSIUS) AND IN A MOIST CONDITION FOR A MINIMUM OF 7 DAYS AFTER PLACEMENT UNLESS OTHERWISE ACCEPTED BY ARCHITECT (STRUCTURAL ENGINEER). USE OF CURING COMPOUND IS ACCEPTABLE, SUBMIT PRODUCTS DATA.

REINFORCING STEEL

1. REINFORCING STEEL:

A. ALL BARS UNLESS INDICATED OTHERWISE: SHALL BE ASTM A615, GRADE 60 (GRADE 400)

B. BARS TO BE WELDED: ASTM A706, GRADE 60 (GRADE 400)

C. ADDITIONAL REQUIREMENTS FOR BARS, EXCLUDING TIES, IN DUCTILE MOMENT RESISTING FRAMES AND BOUNDARY ELEMENTS IN SHEAR WALLS: NO ADDITIONAL REQUIREMENTS IF ASTM A706, GRADE 60 (GRADE 400) BARS USED. ASTM A615, GRADE 60 BARS MAY BE ARE PERMITTED PROVIDED ACTUAL YEILD STRENGTH BASED ON MILL TESTS DOES NOT EXCEED SPECIFIED YIELD STRENGTH BY MORE THAN 18,000 PSI (124 MPa) (RETESTS SHALL NOT EXCEED THIS VALUE BY MORE THAN AN ADDITIONAL (3,000 PSI/ 20MPa) AND RETIO OF ACTUAL ULTIMATE TENSILE STRESS TO ACTUAL YIELD STRESS IS NOT LESS THAN 1.25.

2. WIRE AND SPIRAL REINFORCING:

A. SMOOTH WELDED WIRE FABRIC (WWF.):
ASTM A185, Fy = 65 KSI (450 MPa), FLAT SHEETS ONLY – DO NOT USE ROLLED MESH.
LAP 1 – ½ WIRE SPACES (1 FOOT MINIMUM/ 300mm). OFFSET LAPS IN ADJACENT SHEETS TO AVOID CONTINUOUS LAPS.

B. DEFORMED WIRE STIRRUPS (D4 AND LARGER ONLY): ASTM A497, Fy = 65 KSI (450 MPa)

C. SPIRAL REINFORCING: ASTM A82, GRADE 60 (GRADE 400)

3. LAP SPLICES: PROVIDE CLASS B SPLICES UNLESS INDICATED OTHERWISE. SPLICE #5 (15M) BARS AND LARGER ONLY AT LOCATIONS INDICATED.
A. SPLICES IN WALLS: LOCATE SPLICES IN HORIZONTAL BARS AT WELL-STAGGERED LOCATIONS. DO NOT SPLICE VERTICAL BARS EXCEPT AT HORIZONTAL SUPPORTS SUCH AS FLOOR AND ROOF DIAPHRAGMS.

4. MIN. CLEARANCES BETWEEN PARALLEL REINFORCING STEEL INCLUDING DISTANCE BETWEEN SETS OF SPLICED BARS: 1 INCH (25mm) OR 1 BAR DIAMETER, WHICHEVER IS GREATER. 1-1/2 (40mm) INCHES OR 1-1/2 BAR DIAMETERS, SHICHEVER IS GREATER, AT COLUMNS, PEIORS, AND PILASTERS ONLY. FOR BUNDLED BARDS, MINIMUM CLEAR DISTANCES BETWEEN UNITS OF BUNDLED BARS SHALL BE SAME AS SINGLE BARS EXCEPT BAR DIAMETER IS DERIVED FROM EQUIVALENT TOTAL AREA OF BUNDLE.

5. MINIMUM CONCRETE COVERAGE: PLACE BARS AS NEAR TO CONCRETE SURFACE AS THE FOLLOWING MINIMUM COVERAGE PERMIT, UNLESS NOTED OTHERWISE:

	LOCATE AT CENTER OF SLAB
A. SLAB ON GRADE	
B. SLAB SUPPORTING EARTH ABOVE	1-1/2 INCHES (40mm) FROM TOP
C. FORMED CONCRETE IN CONTACT WITH EARTH	2 INCHES (50mm)
D. CONCRETE POURED AGAINST EARTH (UNFORMED)	3 INCHES (75mm)
E. WALLS ABOVE GRADE, EXPOSED TO WEATHER	2 INCHES (50mm)
F. WALLS ABOVE GRADE, NOT EXPOSED TO WEATHER	¾ INCH (20mm)
G. COLUMNS (CLEAR TO FACE OF TIES)	1-1/2 INCHES (40mm)
H. BEAMS (CLEAR TO FACE OF TIES)	1-1/2 INCHES (40mm)
I. STUCTURAL SLABS (TOP AND BOTTOM)	1 INCH (25mm)

6. DOWELS AT CONSTRUCTION JOINTS: PROVIDE DOWELS MATCHING SIZE AND QUANTITY OF REINFORCING STEEL INTERRUPTED AT CONSTRUCTION JOINTS, UNLESS DETAILED OTHERWISE.

7. PLACEMENT OF BARS IN WALLS: PLACE VERTICAL BARS CLOSEST TO WALL SURFACES AT CURTAINS CONTAINING VERTICAL AND HORIZONTAL BARS OF THE SAME SIZE. IN CURTAINS WHICH VERTICAL AND HORIZONTAL BARS ARE OF DIFFERENT SIZES OR SPACING, PLACE LAYER WITH MOST STEEL AREA CLOSEST TO NEAR WALL SURFACE.

8. BARS TERMINATING AT WALLS, COLUMNS, BEAMS, AND FOUNDATIONS: EXTEND BARS TO WITHIN 2 INCHES (50mm) (3 INCHES (75mm) AT CONCRETE POURED AGAINST EARTH) OF AR FACE OF WALL, COLUMN, BEAM, OR FOUNDATION AND PROVIDE STANDARD ACI 90-DEGREE HOOK UNLESS DETAILED OTHERWISE.

9. BARS INTERUPTED BY STRUCTURAL STEEL: EXTEND BARS TO WITHIN 2 INCHES (50mm) OF STEEL FACE AND PROVIDE STANDARD ACI 90-DEGREE HOOK UNLESS DETAILED OTHERWISE.

10. WELDING: AWS D1.4, EXCEPT AS MODIFIED BY APPLICABLE CODE STANDARD 19-1.
A. ACCEPTABLE STEEL FOR WELDING: ASTM A706. IF WELDING REIFORCING STEEL OTHER THAN A706 IS DESIRED, SUBMIT PROPOSED PROCEDURE, INDICATING CONFORMANCE TO APPLICABLE CODE AND REQUIREMENTS OF GOVERNING CODE AUTHORITY, TO ARCHITECT (STRUCTURAL ENGINEER) FOR ACCEPTANCE AND TO GOVERNING CODE AUTHORITY FOR APPROVAL PRIOR TO EXECUTION.
B. WELDER CERTIFICATION: GOVERNING CODE AUTHORITY.

11. BENDING: BEND COLD, DO NOT FIELD-BEND REIFORCING STEEL BARS EMBEDDED IN CONCRETE

CAST-IN-PLACE CONCRETE

1. SPECIFIED COMPRESSIVE STRENGTH OF MASONRY (f'm): 1,000 PSI (7 MPa) TYPICAL UNLESS NOTES OTHERWISE.

2. VERIFY SPECIFIED COMPRESSIVE STRENGTH OF MASONRY (f'm): PROVIDE MASONRY PRISM TESTING PER ICB-2105.2.2.2 BEFORE AND DURING CONSTRUCTION UNLESS. FULL ALLOWABLE STRESSES ARE USED IN DESIGN. SUBMIT MASONRY PRISM DATA FOR EACH TYPE AND COMPRESIVE STRENGTH OF MASONRY REQUIRED. CORE TESTING MAY BE REQUIRED. IF PRISM TEST FAIL, MORTAR AND GROUT SHALL BE TESTED PER 2105.

3. CONCRETE BLOCK: SHALL CONFORM TO LOCAL STANDARDS & NOTE #1.

4. PORTLAND CEMENT FOR MORTAR AND GROUT: ASTM C150, TYPE I OR II. USE OF MASONRY CEMENT OR PLASTIC CEMENT IS NOT PERMITTED.

5. AGGREGATES FOR MORTAR AND GROUT:

A. AGGREGATES FOR MORTAR: ASTM C144. (OR ACCEPTABLE EQUIVALENT TO LOCAL STANDARDS)
B. AGGREGATES FOR GROUT: C404, COARSE TYPE. (OR ACCEPTABLE EQUIVALENT TO LOCAL STANDARDS)

6. MORTAR: ASTM C270, TYPE S. MIX IN PROPORTIONS ACCORDING TO APPLICABLE CODE SECTION 2.6A, TYPE S.

7. GROUT: ASTM C476 (OR SIMILIAR LOCAL STANDARDS) HOWEVER, IN NO CASE SHALL GROUT COMPRESSIVE STRENGTH BE LESS THAN 200 PSI (1380 kPa) AT 28 DAYS.

8. REIFORCING STEEL: REIFORCING STEEL SECTION OF GENERAL NOTES UNLESS INDICATED OTHERWISE.

9. COMPOSITE MASONRY WALL PENETRATION SUBITTAL: SUBMIT FOR EACH WALL INDICATING SIZE AND LOCATION FOR EACH WALL PENETRATION AND OPENING AS NECESSARY BY AFFECTED TRADES. SUBMIT TOGETHER WITH APPROPRIATE STEEL SHOP DRAWINGS. SUBMIT WRITTEN STATEMENT FROM SPECIAL INSPECTOR THAT NO ADDITIONAL PENETRATIONS OR OPENINGS WERE ADDED TO THOSE SHOWN IN PENETRATION SUBMITTAL.

10. STEEL SPEICES: LAP REIFORCING STEEL AT SPLICES A MINIMUM OF 48 BAR DIAMETERS, EXCEPT DOWELS IN FOOTINGS AT BASE OF WALLS SHALL SPLICE A MINIMUM OF 72 BAR DIAMETERS, UNLESS NOTED OTHERWISE. WHERE MINIMUM CLEAR DISTANCE BETWEEN BARS AT ADJACENT SPLICES IS 3 INCHES (75mm) OR LESS, INCREASE LAP LENGTH 30 PERCENT UNLESS SLPICES ARE STAGGERED AT LEAST 24 BAR DIAMETERS.

11. DOWELS FOR WALLS, COLUMNS, PILASTERS, AND PEIRS: MATCH SIZE AND SPACING FOR VERTICAL REINFORCING STEEL, UNLESS NOTED OTHERWISE. SET DOWELS TO ALIGN WITH CELLS CONTAINING STEEL.

12. MINIMUM REIFORCING STEEL CLEARANCES:

A. MINIMUM CLEARANCES BETWEEN REIFORCING AND OUTSIDE FACE OF MASONRY: 2 INCHES (50mm) EXCEPT IN NO CASE SHALL CLEARANCE BE LESS THAN 2-1/2 (60mm) BAR DIAMETERS.
B. MINIMUM CLEARANCE BETWEEN REIFORCING AND INSIDE FACE OF GROUT CELL: ½ INCH (12mm).
C. MINIMUM CLEARANCE DISTANCES BETWEEN PARALLEL REIFORCING: 1 INCH (25mm) OR NORMAL BAR DIAMETER, WHICHEVER IS LESS. INCREASE TO 1-1/2 INCHES (40mm) OR 1-1/2 TIMES MORMAL BAR DIAMETER, WHICHEVER IS LESS, AT COLUMNS, PILASTERS, AND PIERS ONLY.

13. PLACEMENT: SET COURSES IN RUNNING BOND PATTERN UNLESS INDICATED OTHERWISE. SET CELLS IN VERTICAL ALIGNMENT. PROVIDE FLUSH MORTAR JOINTS TO SURFACES TO RECEIVE WATERPROOFING OR DAMP-PROOFING.

14. GROUTING: GROUT SOLID ALL CELLS THAT CONTAIN REINFORCING BARS, VIBRATE GROUT IN CELLS.

A. GROUT HEIGHT LIMITS: APPLICABLE CODE TABLE 21-C (OR 1.5 METER IN HEIGHT
B. HORIZONTAL CONSTRUCTION JOINTS: HOLD GROUT 1 ½ INCHES (40mm) BELOW TOP OF MASONRY UNIT IF WORK IS STOPPED ONE HOUR OR LONGER.
C. GROUT COVER AROUND STEEL, ANCHOR BOLTS AND INSERTS PENETRATING MASONRY SHELL: 1 INCH (25mm) MINIMUM.

15. HORIZONTAL BARS TERMINATING AT WALL ENDS AND OPENING JAMBS: EXTEND BARS TO WITHIN 2 INCHES (50mm) OF END OF WALL AND PROVIDE STANDARD ACI 90-DEGREE HOOK UNLESS DETAILED OTHERWISE.

STRUCTURAL INDEX

00001 - FIRE (Joseph Moon)
Demolition construction **documents** must show sewer lateral(s) location(s) with detailing for lateral termination. Detailing to show lateral swept and extended upward with cleanout cap , in a vault.

e-PlanSoft™

version 1

NOT FOR CONSTRUCTION

Revisions

△		
△		
△		
△		
△		

GENERAL NOTES

S-01